

Comparative Study of Soil Models for Deep Excavation Analysis in Soft Soils Using the Finite Element Method: A Case Study of the Muara Tawar Coastal Project with Field Monitoring Data

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Abstract

The construction of underground structures at the Muara Tawar Combined Cycle Power Plant, located on a coastal site with very soft clay soil, presents significant geotechnical challenges. This study investigates the effectiveness of temporary sheetpile and strutting retaining walls during the excavation of the Steam Turbine Generator (STG) 4 basement, which is adjacent to existing structures. Utilizing PLAXIS 2D, an advanced Finite Element Method (FEM) software, we modeled and analyzed the soil-structure interaction to optimize the design and construction approach by comparing several soil constitutive models, including Mohr-Coulomb, Soft Soil, and Hardening Soil. Numerical analyses, validated against field monitoring data, demonstrate that the proposed sheetpile and strutting system effectively prevents soil collapse during construction, yielding satisfactory safety factors and acceptable deflection values. By comparing the numerical results with actual monitoring data, we determined which model accurately predicts soil behavior for this specific construction scenario. The results indicate that complex soil constitutive models, such as Soft Soil and Hardening Soil, provide more accurate predictions compared to the Mohr-Coulomb model.

Keywords: Soil model comparation, finite element method, soft soil, deep excavation, sheetpile

Abstrak

Pembangunan struktur bawah tanah di Pembangkit Listrik Tenaga Gas dan Uap Muara Tawar, yang terletak di daerah pesisir dengan tanah lempung yang sangat lunak, menghadirkan tantangan geoteknik yang signifikan. Studi ini menyelidiki efektivitas dinding penahan sementara berupa sheetpile dan strutting selama penggalian ruang bawah tanah Steam Turbine Generator (STG) 4, yang berdekatan dengan struktur eksisting. Dengan menggunakan PLAXIS 2D, perangkat lunak Metode Elemen Hingga (FEM) yang canggih, kami memodelkan dan menganalisis interaksi tanah-struktur untuk mengoptimalkan pendekatan desain dan konstruksi dengan membandingkan beberapa model konstitutif tanah, termasuk Mohr-Coulomb, Soft Soil, dan Hardening Soil. Analisis numerik, yang divalidasi dengan data pemantauan lapangan, menunjukkan bahwa sistem sheetpile dan strutting yang diusulkan secara efektif mencegah keruntuhan tanah selama konstruksi, menghasilkan faktor keamanan yang memadai dan nilai defleksi yang dapat diterima. Dengan membandingkan hasil numerik dengan data pemantauan aktual, kami menentukan model mana yang secara akurat memprediksi perilaku tanah untuk skenario konstruksi spesifik ini. Hasil penelitian menunjukkan bahwa model konstitutif tanah yang kompleks, seperti Soft Soil dan Hardening Soil, memberikan prediksi yang lebih unggul dibandingkan model Mohr-Coulomb.

Kata Kunci: Komparasi model tanah, metode elemen hingga, tanah lunak, galian dalam, sheetpile

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1. Introduction

The Muara Tawar Combined Cycle Power Plant (CCPP) 650 MW, is part of the National Strategic Project built in the Bekasi Coastal Area, West Java for bolstering national energy security and supporting Indonesia's economic growth. However, constructing CCPP in coastal areas often presents significant geotechnical challenges. The Muara Tawar CCPP site, for instance, sits on a geological formation dominated by very soft clay. This type of soil, characterized by low shear strength, high compressibility, and high water content(Hausmann, 1990; Bjerrum, 1967), is highly susceptible to deformation and can potentially lead to stability issues in construction (Holtz et al., 2011). This necessitates specialized handling and poses a considerable challenge for geotechnical engineers in designing foundations and substructures (Leroueil et al., 1985; Mitchell & Soga, 2005). The presence of very soft clay at the Muara Tawar site necessitates careful consideration and innovative solutions to ensure the stability and safety of the power plant's bottom structures, particularly for the Main Building, during the construction stage, especially given the need for deep excavation.

One common method to address the challenges of soft soil is using sheetpiles as retaining walls. Sheetpiles function as temporary retaining structures during the construction period, especially in excavation works. To enhance their strength and stability, sheetpiles are often combined with strutting as support (Sabatini et al, 1999). These retaining wall system preferably utilize elastoplastic modelling, as elastoplastic support systems provide a more realistic estimation of excavation stability compared to elastic systems (Do et al, 2016). The bottommost one or two installed struts and wall are critical components that can trigger progressive collapse if they fail (Cheng et al, 2024). Proper design and installation of sheetpile walls are essential to prevent excessive deformation and maintain the integrity of the excavation, especially in challenging soil conditions like those found at the Muara Tawar site.

This research focuses on analyzing the performance of steel sheetpile walls with multi-strut configurations

used as earth retaining structures using Finite Element Method (FEM). FEM is chosen because FEM can reproduce the measurements since arching effect is allowed within the soil (Cheng et al, 2024). The study aims to evaluate the effectiveness of these walls in mitigating lateral earth pressure and ensuring the stability of excavation slopes, while also safeguarding the safety of nearby structures. The analysis will employ three soil models—Mohr- Coulomb (MC), Soft Soil (SS), and Hardening Soil (HS)—to simulate soil behavior. From the past study, it finds that Advanced Soil Model is probably more accurate than Mohr-Coloumb soil model (Hamonangan & Syahputra, 2023; Nawir et al, 2017; Schiena et al, 2024). By comparing the results obtained from these models with field monitoring data, the research seeks to determine which model offers the most realistic prediction of the wall's performance. This finding will contribute valuable insights for optimizing the design and implementation of sheetpile walls in similar geotechnical conditions. material based on a single-dimension approach

2. Methodology

This research analyzes the stability of deep excavations in soft soil at the Muara Tawar coastal area using the finite element method. The focus is on evaluating the performance of sheetpile retaining walls with bracing reinforcement in the Steam Turbine Building (STB) #4 area. The analysis utilizes Plaxis 2D software and compares safety factors, bending moments, and deflections against field monitoring data using three different soil models: Mohr-Coulomb, Soft Soil, and Hardening Soil.

This study analyzed deep excavation in soft soil through a comprehensive literature review and extensive data collection (including soil investigation, field tests, and monitoring data). Plaxis 2D was used to evaluate temporary retaining wall design strength and compare safety factors for different sheetpile types and soil models, ultimately providing recommendations for addressing deep excavation challenges. The sheetpile design for the excavation area in the Muara Tawar CCPP Block 2,



Figure 1. Study location

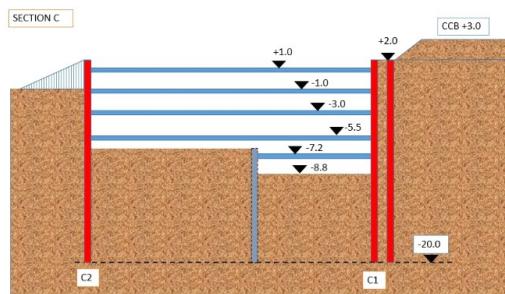


Figure 2. Temporary support during construction

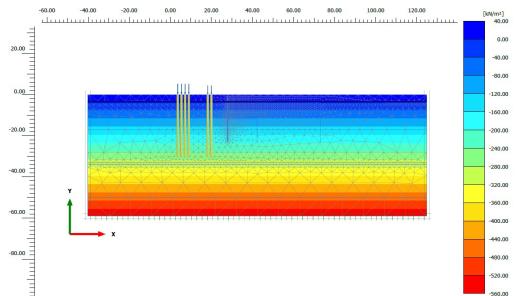


Figure 4. Input data flow condition of the FE analysis

3, and 4 Add-on 650 MW project is based on soil investigation data from Borehole (BH)-05 and Cone Penetration Test (CPT)-05 (Figure 1), considered closest to the analysis location.

2.1 Location

The study areas for this research are located within Muara Tawar CCPP Project 650 MW, Jalan Muara Tawar No 1, Kab. Bekasi, West Java, Indonesia.

2.2 Numerical model

The analysis conducted using the FEM software Plaxis 24.1 focuses on the deepest excavation area directly adjacent to the Central Control Building (CCB), with a detailed cross-section shown in Figure 2.

The planned excavation procedure is as follows:

- Lower the ground level from +3.00 to +2.00.
- Install sheetpiles to a depth of 22 m (12 m + 10 m) reaching elevation -20.00. Use a double layer of sheetpiles on the side facing the Central Control Building (CCB) and a single layer on the opposite side.
- Excavate 1 m of soil to reach elevation +1.00 and install a double layer of H-beam 300 struts (blue).
- Repeat step 3 for every subsequent excavation depth of +/- 2 m, installing struts at elevations -1.00, -3.00, and -5.50.
- Excavate to elevation -7.00 and install additional sheetpiles to reach a depth of -20.00.
- Continue excavation to elevation -7.20 m and install a 2-layer H-beam 300 strutting system.
- Excavate to the final planned elevation of -8.80 m.

The finite element analysis of this paper employs three soil models—Mohr-Coulomb, Soft Soil, and Hardening

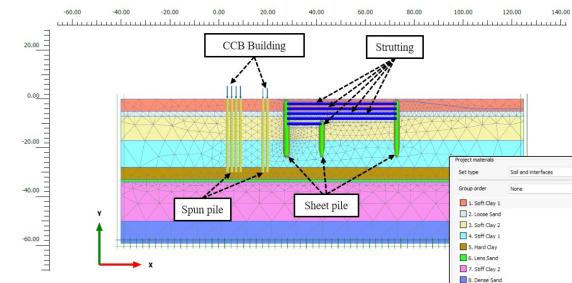


Figure 3. Geometry data input of the FE analysis

Table 1. Plaxis modelling notes

No	Items	Model
1	Soil Model 1	Soft Soil (SS)
2	Soil Model 2	Hardening Soil (HS)
3	Soil Model 3	Mohr – Columb (MS)
4	Sheetpile	Plate
5	Spun Pile CCB	Embedded Beam
6	Bangunan CCB	Point Load
7	Strutting	Node to Node Anchor

Soil—to compare safety factors for two common U-shaped sheet pile types (Type 2 and Type 3, conforming to BS EN 10249 for cold-formed non-alloy steel) available in Indonesia, which exhibit key differences in dimensions and yield strength. Displacement output for each soil model is also analyzed. Subsequently, safety factor and displacement analysis data are compared with actual field monitoring results. Based on the described sheet pile installation plan, the excavation process was modeled using the finite element software Plaxis 2D 24, as shown in Figures 3 and Figure 4.

2.3 Model parameter

The modeling steps for Mohr-Coulomb, Hardening Soil, and Soft Soil are identical. The primary difference lies in the soil parameters used. The Soft Soil model utilizes modified compression index (λ) and modified swelling index (κ) parameters that could be substituted using Compression Index (Cc) and Swelling Index (Cs). In contrast, the Mohr-Coulomb model uses modulus (E) and Poisson's ratio (ν) values. The Hardening Soil model employs modulus values obtained from triaxial and consolidation tests (Eur, E50, and Eoed).

To simulate the sheetpile wall itself, two types were considered: Type II and Type III U shape sheetpile walls (with distinct material properties), H-beam 300 struts for lateral support, and 600 mm diameter spun piles to represent the existing Central Control Building foundation. Soil and material properties shown in Tables 2 - Tables 7).

To accurately represent the existing Central Control Building (CCB) on the east side of the excavation, we incorporated the ultimate loads transferred to each pile (CCB building load + pile cap load). These loads were then evenly distributed across all piles. Building load data was applied as detailed in Table 8.

Table 2. Inputted Soil Properties of the Mohr Coulomb Model

Depth (m)	Soil Model	Drainage	Soil	SPT	γ_d (kN/m ³)	γ_{sat} (kN/m ³)	E (kN/m ³)	c' (kN/m ²)	ϕ' degree	e	u	k (m/day)
0 – 5	MC	Undrained A	Clay	1	11	16	2500	1	17	1.29	0,3	10^{-10}
5 – 7	MC	Drained	Sand	11	12	17	11000	1	20	1.25	0,2	10^{-4}
7 – 18	MC	Undrained A	Clay	1	8	15	3300	1	17	1.99	0,3	10^{-10}
18 – 27	MC	Undrained A	Clay	11	12	18	9000	20	25	1.28	0,3	10^{-10}
27 – 32	MC	Undrained A	Clay	41	20	20	20000	25	36	0.69	0,3	10^{-9}
32 – 33	MC	Drained	Sand	60	21	23	75000	2	37	0.85	0,3	10^{-4}
33 – 50	MC	Undrained A	Clay	14	9	15	9000	20	25	1.78	0,3	10^{-10}
50 – 59	MC	Drained	Sand	35	19	21	50000	2	37	1.00	0,3	10^{-7}

Table 3. Inputted Soil Properties of the Soft Soil Model

Depth (m)	Soil Model	Drain.	Soil	SPT	γ_d (kN/m ³)	γ_{sat} (kN/m ³)	E_{50} (kN/m ³)	E_{ur} (kN/m ³)	c' (kN/m ²)	ϕ' degree	e	u	k (m/day)	C_c	C_s
0 – 5	SS	Undr. A	Clay	1	11	16	-	-	1	17	1.29	0,3	10^{-10}	0,21	0,05
5 – 7	HS	Drained	Sand	11	12	17	11000	33000	1	20	1.25	0,2	10^{-4}	-	-
7 – 18	SS	Undr. A	Clay	1	8	15	-	-	1	17	1.99	0,3	10^{-10}	0,22	0,05
18 – 27	HS	Undr. A	Clay	11	12	18	9000	27000	20	25	1.28	0,3	10^{-10}	-	-
27 – 32	HS	Undr. A	Clay	41	20	20	20000	60000	25	36	0.69	0,3	10^{-9}	-	-
32 – 33	HS	Drained	Sand	60	21	23	75000	225000	2	37	0.85	0,3	10^{-4}	-	-
33 – 50	HS	Undr. A	Clay	14	9	15	9000	27000	20	25	1.78	0,3	10^{-10}	-	-
50 – 59	HS	Drained	Sand	35	19	21	50000	150000	2	37	1.00	0,3	10^{-7}	-	-

Table 3. Inputted Soil Properties of the Soft Soil Model

Depth (m)	Soil Model	Drainage	Soil	SPT	γ_d (kN/m ³)	γ_{sat} (kN/m ³)	E_{50} (kN/m ³)	E_{ur} (kN/m ³)	c' (kN/m ²)	ϕ' degree	e	u	k (m/day)
0 – 5	HS	Undrained A	Clay	1	11	16	2500	7500	1	17	1.29	0,3	10^{-10}
5 – 7	HS	Drained	Sand	11	12	17	11000	33000	1	20	1.25	0,2	10^{-4}
7 – 18	HS	Undrained A	Clay	1	8	15	3300	9900	1	17	1.99	0,3	10^{-10}
18 – 27	HS	Undrained A	Clay	11	12	18	9000	27000	20	25	1.28	0,3	10^{-10}
27 – 32	HS	Undrained A	Clay	41	20	20	20000	60000	25	36	0.69	0,3	10^{-9}
32 – 33	HS	Drained	Sand	60	21	23	75000	225000	2	37	0.85	0,3	10^{-4}
33 – 50	HS	Undrained A	Clay	14	9	15	9000	27000	20	25	1.78	0,3	10^{-10}
50 – 59	HS	Drained	Sand	35	19	21	50000	150000	2	37	1	0,3	10^{-7}

Table 5. Inputted properties of U - sheetpile

Parameter	Type III	Type 2
Cross Sectional Area, A (mm ² /m)	19000	15250
Inertia Moment, I (mm ⁴ /m)	168000000	87400000
Modulus, E (mPa)	200000	200000
Section Modulus (mm ³ /m)	134000	874000
w (kN/m/m)	1.50	1.20
EA (kN/m)	3820000	3060000
EI (kNm ² /m)	33600	17480
u	0.30	0.30

Table 7. Properties of spun pile

Parameter	Spunpile D-600
Material Type	Elastoplastic
Gamma (kN/m ³)	24
L spacing (m)	1.5
Diameter (m)	0.6
Thickness (m)	0.1

Table 8. Inputted properties of spun piles

Pile Cap	Resultant	Pile Qty	Load each Pile
F2	214.2 kN	4	53.55 kN
FC2	508.5 kN	8	63.56 kN

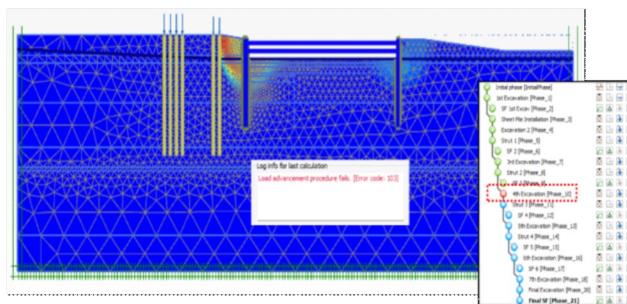


Figure 5. Mohr-Coulomb soil modeling using type 2 sheetpile

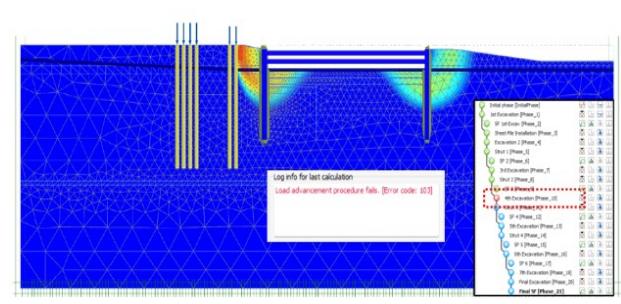


Figure 6. Soft soil modeling using type 2 sheetpile

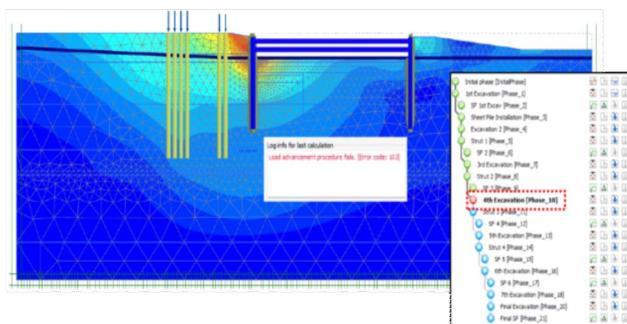


Figure 7. Hardening soil modeling using type 2 sheetpile

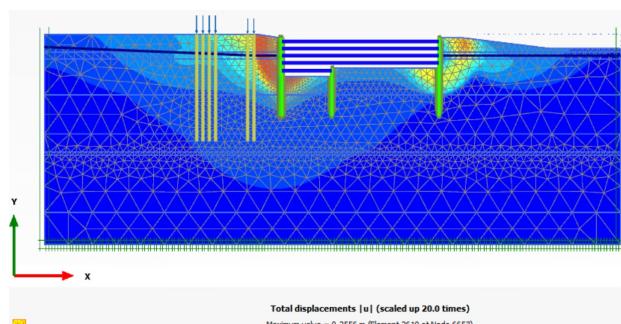


Figure 8. Mohr-Coulomb soil modeling using type 3 sheetpile

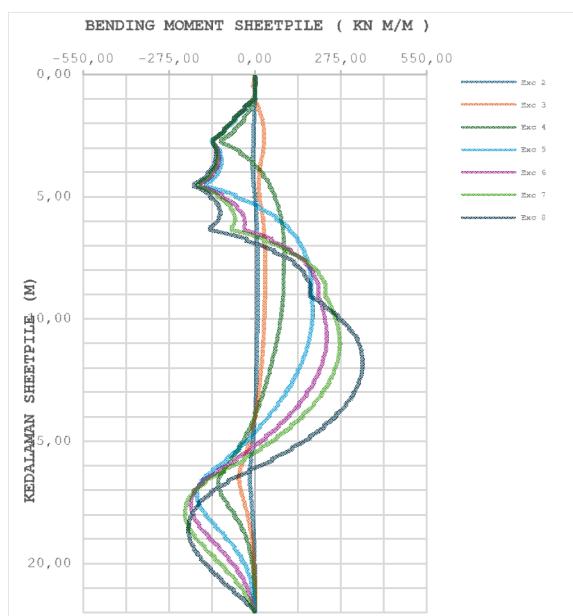


Figure 9. Calculated bending moments for sheetpile 3 - Mohr-Coulomb

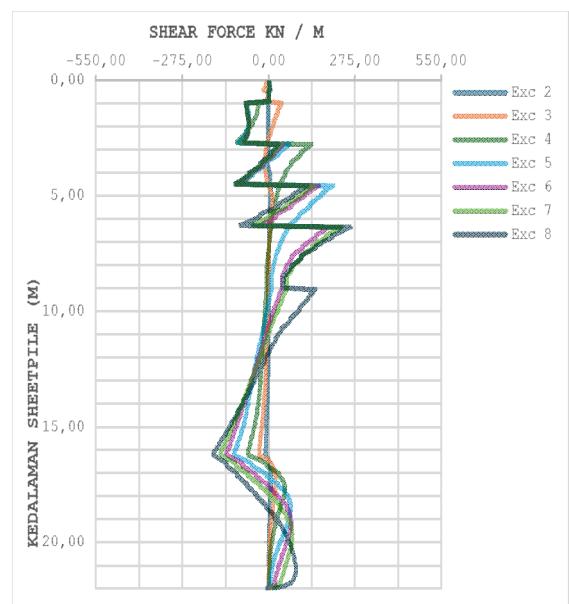


Figure 9. Calculated bending moments for sheetpile 3 - Mohr-Coulomb

3. Result and Discussion

The analysis conducted using a professional FE software called Plaxis2D 24.1 focuses on the deepest excavation area directly adjacent to the Central Control Building (CCB), with a result as shown:

3.1 Sheetpile type 2

In the finite element simulations using Type 2 sheetpiles, numerical analysis employing three different soil models consistently showed slope failure during the excavation process, particularly at the 4th excavation stage (Figures 5 to Figures 7). This indicates that implementing Type 2

sheetpiles is not technically feasible. Therefore, the research focus shifted to utilizing Type 3 sheetpiles.

3.2 Sheetpile type 3 Mohr-Coulomb model

The analysis results show that excavation using Type 3 sheetpiles with H-Beam 300 strutting and the Mohr-Coulomb soil model yields a safety factor (SF) of 1.69 and total displacement of 0.255 m (Figure 8). The maximum bending moment is 345.4 kN m/m, while the minimum bending moment is -213.1 kN m/m (Figure 11). The maximum shear force is 261.8 kN/m, and the minimum shear force is -176.7 kN/m (Figure 11).

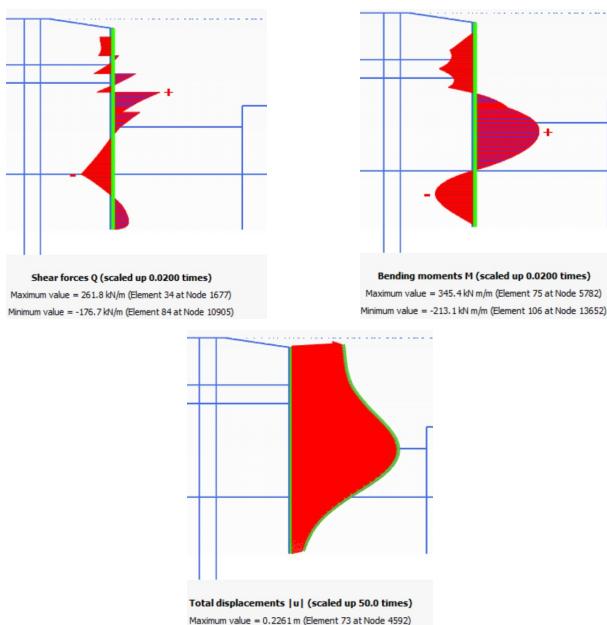


Figure 11. Ultimate values for bending moment, shear force, and displacement sheetpile 3 - Mohr-Coulomb

Based on the analysis of the bending moment and shear force envelopes (Figures 9 and Figures 10), it was identified that the maximum values occur at the final excavation stage (8th excavation). Therefore, the ultimate bending moment and shear force values used in the design are those that occur at the 8th excavation stage

3.3 Sheetpile type 3 soft soil model

The analysis results indicate that excavation using Type 3 sheetpiles with H-Beam 300 strutting and the Soft Soil model yields a safety factor (SF) of 1.64 and

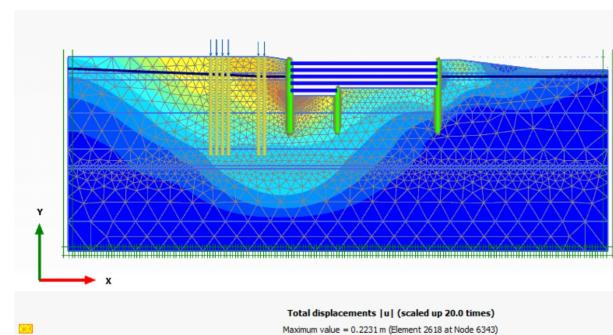


Figure 12. Soft soil modeling using type 3 sheetpile

a maximum displacement of 0.223 m (Figure 12). The maximum bending moment is 295.5 kN m/m, while the minimum bending moment is -197.7 kN m/m (Figure 15). The maximum shear force is 273.4 kN/m, and the minimum shear force is -134.1 kN/m (Figure 15).

Based on the analysis of the bending moment and shear force envelopes (Figures 13 and 14), it was determined that the maximum values occur at the final excavation stage (8th excavation). Therefore, the ultimate bending moment and shear force values used in the design are those corresponding to the 8th excavation stage.

3.4 Sheetpile type 3 hardening soil model

The analysis using Type 3 sheetpiles, H-Beam 300 strutting, and the Hardening Soil model shows a safety factor (SF) of 1.95 and a maximum displacement of 0.22 (Figure 16) m. The maximum and minimum bending moments are 271.3 kN m/m and -216.5 kN m/m (Figure 19). Respectively, the maximum shear force is 282.0 kN/m, while the minimum is -118.1 kN/m (Figure 19).

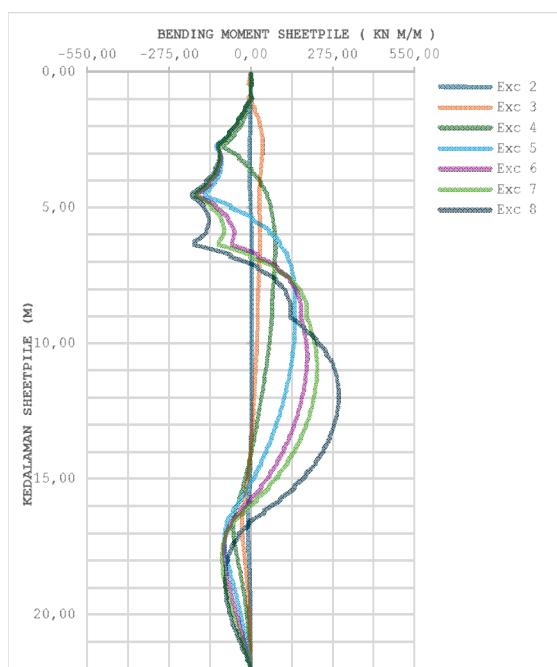


Figure 13. Calculated bending moments for sheetpile 3 – soft soil

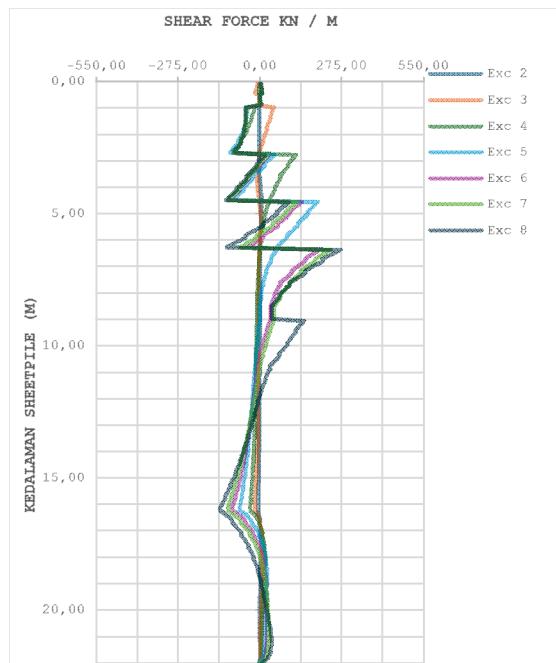


Figure 14. Calculated shear forces for sheetpile 3 – soft soil

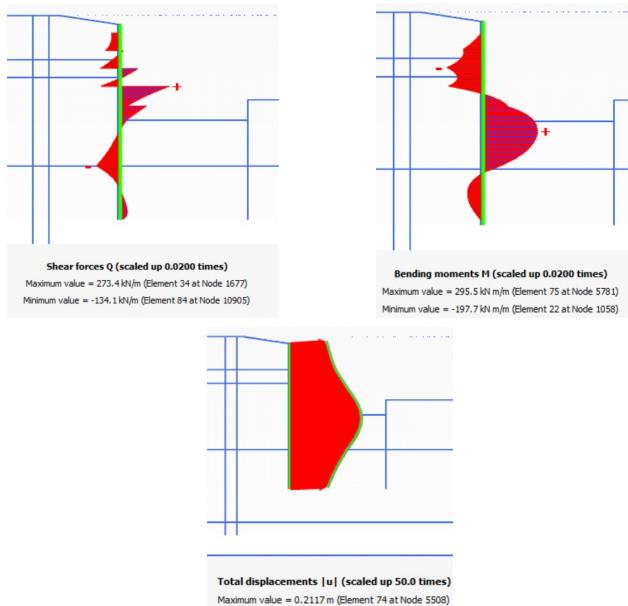


Figure 15. Ultimate values for bending moment, shear force, and displacement sheetpile 3 – soft soil

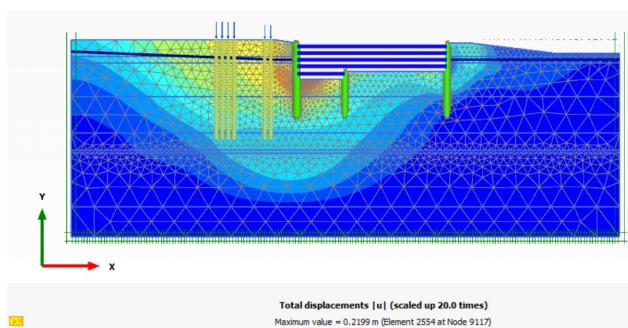


Figure 16. Hardening soil modeling using type 3 sheetpile

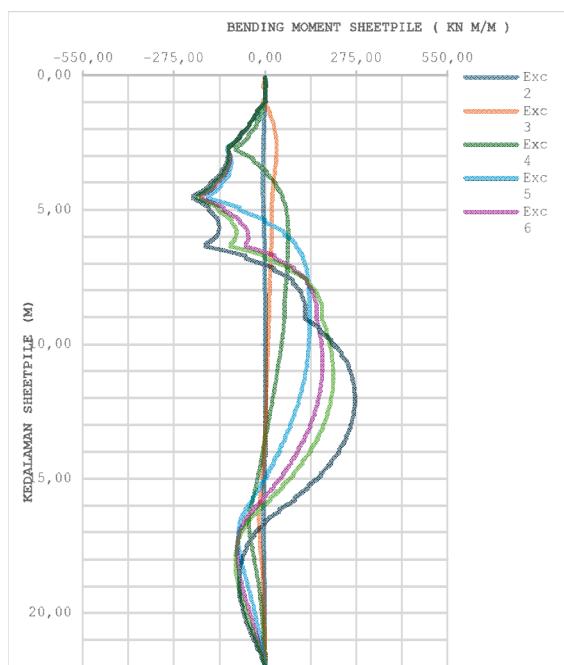


Figure 17. Calculated bending moments for sheetpile 3 – hardening soil

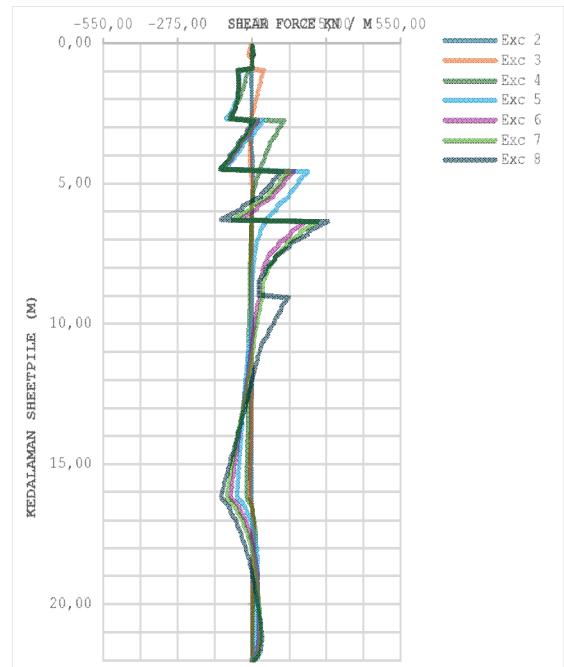


Figure 18. Calculated shear forces for sheetpile 3 – soft soil

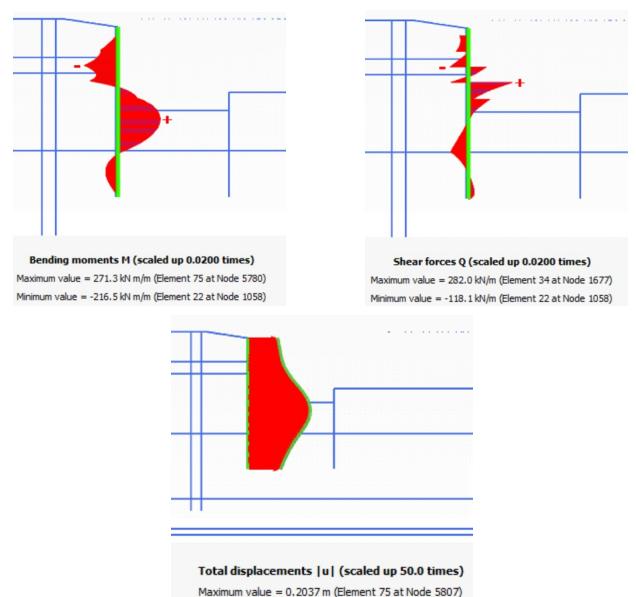


Figure 19. Ultimate values for bending moment, shear force, and displacement sheetpile 3 – hardening soil

Analysis of the bending moment and shear force envelopes (Figure 17 and Figure 18) indicates that maximum values occur at the final (8th) excavation stage. Therefore, design calculations utilize the ultimate bending moment and shear force values corresponding to this stage.

3.5 Discussion

In this study, monitoring was conducted at three monitoring points: point A, point B, and point C (Figure 20). However, due to implementation limitations, data was only collected at the top of the

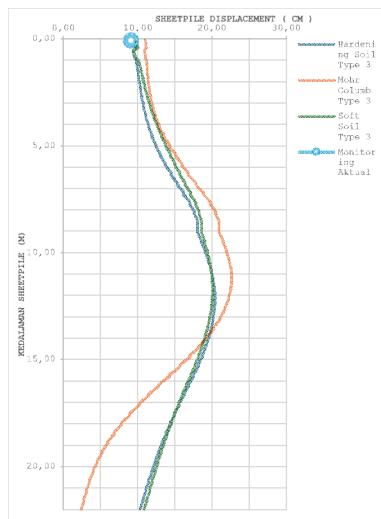


Figure 20. Sheetpile displacement

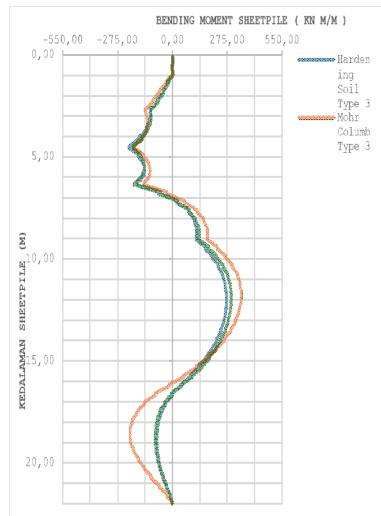


Figure 22. Calculated bending moments of Hardening Soil vs Mohr Coulomb for type 3

Remark :

1. In the analysis using sheetpile type 2, a collapse occurred resulting in the bending moment not being formed.
2. The red dashed line represents the moment capacity of sheetpile type 3, which is 528 kN/m.

Table 9. Properties of strutting beam

SP Type	Soil Model	Displ. (mm)	Tip Displ. (mm)	Remark
Type 3	SS	211	97	-
	HS	203	95	-
	MC	226	110	-
Type 2	SS	-	-	Failure
	HS	-	-	Failure
	MC	-	-	Failure
Monitoring		-	91	-

Table 11. Resume bending moment

SP Type	Soil Model	Maximum (kN m/m)	Minimum (kN m/m)	Remark
Type 3	SS	295.5	-197.7	-
	HS	271.3	-216.5	-
	MC	345.4	-213.1	-
Type 2	SS	-	-	Failure
	HS	-	-	Failure
	MC	-	-	Failure

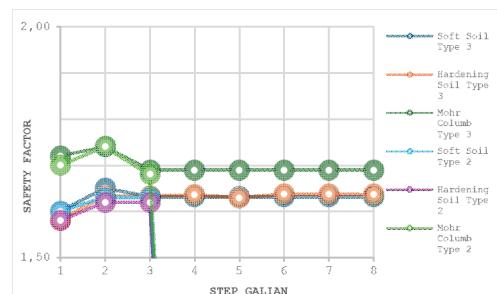


Figure 21. Calculated safety factor values

Note: When using sheetpile type 2, collapse occurred at the 4th excavation step, resulting in a safety factor of 0 after reaching that depth.

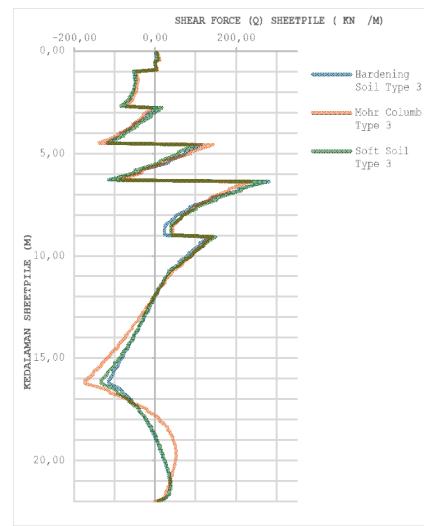


Figure 23. Calculated shear forces of Hardening Soils vs Mohr Coulomb vs Soft Soils for Type 3

Remark :

1. In the analysis using sheetpile type 2, a collapse occurred resulting in the shear strength not being formed.

Table 10. Resume safety factor Plaxis 2D

No	Exc. Step	SS Type 3	HS Type 3	MC Type 3	SS Type 2	HS Type 2	MC Type 2
1	1	1.60	1.58	1.72	1.60	1.58	1.70
2	2	1.65	1.63	1.74	1.63	1.62	1.74
3	3	1.63	1.63	1.69	1.63	1.62	1.68
4	4	1.63	1.64	1.69	-	-	-
5	5	1.63	1.63	1.69	-	-	-
6	6	1.63	1.64	1.69	-	-	-
7	7	1.63	1.64	1.69	-	-	-
8	8	1.63	1.64	1.69	-	-	-

Table 12. Resume shear strength

SP Type	Soil Model	Maximum (kN/m)	Minimum (kN/m)	Remark
Type 3	SS	273.4	-134.1	
	HS	282.0	-118.1	
	MC	261.8	-176.7	
Type 2	SS	-	-	Failure
	HS	-	-	Failure
	MC	-	-	Failure

sheetpile, specifically at elevation -1.00 (shown in **Table 9**, based on **Figure 20**).

The envelope safety factor data for each soil model is shown in **Figure 21** based on **Table 10**.

Bending Moment Data shown in **Table 11** and **Figure 22** (based on **Figure 9**; **Figure 13**; and **Figure 17**).

Shear Strength Result Data shown in **Table 12** and **Figure 23** (based on **Figure 10**; **Figure 14**; and **Figure 18**).

4. Conclusion

The key findings of this research are as follows:

1. The 2D finite element analysis results show that the type 2 sheetpile system with double layer strutting failed at the 4th excavation stage. However, the type 3 sheetpile system with double layer strutting remained stable, with the Mohr-Coulomb soil model yielding the highest Safety Factor (SF) of 1.78, followed by the Hardening Soil and Soft Soil models both at 1.64.
2. Analysis results show that the maximum bending moment, deformation, and shear force occur at the excavation base, consistent with Song et al. (2024), stated that the last one or two struts are the most vulnerable components and can trigger progressive collapse if they fail.
3. The vertical deformation analysis using the finite element method demonstrated the high accuracy of the Hardening Soil model, which predicted deformation within +4 mm of the field monitoring data. The Soft Soil and Mohr-Coulomb models showed deviations of +6 mm and +19 mm, respectively. These results support the findings of Bozkurt et al. (2023), who emphasized the reliability of finite element methods in predicting realistic deformations in deep excavation scenarios. When detailed soil data are available, the use of advanced soil models such as Soft Soil and Hardening Soil is recommended over the Mohr-Coulomb model to achieve greater predictive accuracy.
4. The finite element analysis using the Hardening Soil model predicted a vertical deformation of 95 mm, which is close to the field monitoring data of 91 mm. Although this value slightly exceeds the limit specified in the Indonesian National Standard (SNI 8460:2017), which is 80 mm (or 0.5% of the 16 m sheet pile height), various anticipatory measures implemented during excavation proved effective. These included routine visual monitoring (which revealed no cracks or signs of landslides), effective groundwater drawdown control, restrictions on heavy equipment loads, and thorough inspections of the sheet pile, particularly at the joints. As a result, no signs of excavation failure were observed, and the structure was successfully constructed.
5. A comparison of soil models revealed that advanced models, such as Hardening Soil and Soft Soil, provided superior predictions of sheetpile deflection compared to the Mohr-Coulomb model. This

finding is supported by the research of Nawir et al. (2017), which demonstrated the higher accuracy of the Hardening Soil (Advanced) model. Consequently, when detailed soil data is available, employing these advanced models is recommended to achieve a more precise analysis

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References

Bjerrum, L. (1967). *Engineering Geology of Norwegian Normally-Consolidated Marine Clays as Related to Settlements of Buildings*. Géotechnique, 17(2), 81-118.

Bowles, J. E. (1997). *Foundation Analysis and Design*. The McGraw-Hill, Illinois

Bozkurt, S., Abed, A., & Karstunen, M. (2023). "Finite element analysis for a deep excavation in soft clay supported by lime-cement columns". Computers and Geotechnics, 162, 105687.

BS EN 10249. (1996). *Cold formed sheet piling of non alloy steels*. BSI. London.

Cheng, X., et al. (2024) *Study on the Mechanism and Countermeasures of Progressive Collapse in Deep Excavation Retained by Multilayer Struts*. Structures, 60, 105837.

Do, T.-N. et al. (2016). *A Study of Failure Mechanisms of Deep Excavations In Soft Clay Using The Finite Element Method*. Computers and Geotechnics, 73, 153-163.

Endicott, L.J. (2015). *Design and Construction of Excavations in an Urban Setting – Lessons Learnt from Failures*. International Conference on Geotechnical Engineering, Colombo.

Hamonangan, E.K., & Syahputra, M.Y. (2023). *Perbandingan Hasil Analisis Finite Element untuk Stabilitas dan Penurunan Timbunan dengan Beberapa Soil Model di Lokasi Rencana Pembangunan Jalan Tol Subang*. Jurnal HPJI (Himpunan Pengembangan Jalan Indonesia) Vol. 9 No. 1: 1-8.

Hardiyatmo, H.C. (2002). *Teknik Pondasi 2*. Gadjah Mada University Press, Yogyakarta

Hausmann, M.R. (1990). *Engineering Principles of Ground Modification*, McGraw-Hill. New York.

Holtz, R.D., Kovacs, W.D., & Sheahan, T.C. (2011). *An Introduction to Geotechnical Engineering*. Pearson, New Jersey.

Kementerian Pekerjaan Umum dan Perumahan Rakyat (PUPR). 2002. *Panduan Geoteknik 1 Proses Pembentukan dan Sifat Sifat Dasar Tanah Lunak*. Kementerian PUPR, Jakarta.

Leroueil, S., Tavenas, F., Brucy, F., La Rochelle, P., & Roy, M. (1985) *Behavior of Destructured Natural Clays*. Journal of Geotechnical Engineering, ASCE, 111(6), 751-767.

Mitchell, J.K. & Soga, K. (2005). *Fundamentals of Soil Behavior*. John Wiley & Sons, New Jersey.

Nejar, K. et al., (2022). *Numerical Modelling of a 32 M Deep Excavation in The Suburbs of Paris*. Engineering Structures, 268, 114727.

Nawir, H., Prasetyo, E.B, Apoji, D. (2017). *Excavation Analysis using Stress Path Dependent Soil Parameters*. Regional Conference in Civil Engineering (RCCE). The Third International Conference on Civil Engineering Research (ICCR), Surabaya.

Ou, C-Y. (2006) *Deep Excavation: Theory and Practice*. Taylor & Francis, London

PT. Hutama Karya. (2019). *Dokumentasi Internal Proyek PLTGU Muara Tawar 650 MW*. Bekasi.

Puller, M., (2003). *Deep Excavations: A Practical Manual*. Thomas Telford Publishing, London.

Sabatini, P.J., Pass, D.G., dan Bachus, R.C. (1999). *Geotechnical Engineering Circular No. 4 - Ground Anchors and Anchored Systems*. U.S. Department of Transportation, Federal Highway Administration, Washington, D.C.

Schienna, F. et.al. (2024). *A strain applied method for FEM-2D modelling of TBM tunnels in coarse-grained soils with comparative analysis of case histories*. Tunnelling and Underground Space Technology, 153, 106001.

SNI 8460. (2017). *Persyaratan Perancangan Geoteknik*. Badan Standardisasi Nasional, Jakarta.

Terzaghi, K. Peck, R.B. (1967) *Soil Mechanics in Engineering Practice*, Wiley, NY.